Florida Product Approval Table							
Product Catagory	Sub Catagory	Manufacturer	State Of Florida Approval Number	Approval Date			
ROOFING	FIBERGLASS SHINGLE	CERTAINTEED	FL 10124	2020			
ROOFING	UNDERLAYMENT	OWENS CORNING	FL 15216 R8	2020			
SIDING	HARDI SIDING	JAMES HARDI	FL 13192	2020			
WINDOW	SINGLE HUNG, HORIZ. SLIDER FIXED GLASS	SILVER LINE	FL 14911.5	2020			
EXTERIOR DOOR	INSULATED FIBERGLASS	MASONITE	FL 29847.3	2020			

UNDERLAYMENT USED UNDER ALL NEW ROOFING MATERIAL AS SPECIFIED

706.7.1 Roof decking attachment for site-built single-family residential structures. For site-built single family residential structures the fastening shall be in accordance with Section 706.7.1.1 or 706.7.1.2 as appropriate for the existing construction. 8d nails shall be a minimum of 0.113 inch in diameter and shall be a minimum of 2-1/4 inch long to qualify for the provisions of this section for existing nails regardless of head shape or head diameter. 706.7.1.1 Roof decking consisting of sawn lumber or wood planks up to 12 inches wide and secured with at least two nails (minimum size 8d) to each roof framing member it crosses shall be deemed to be sufficiently connected. Sawn lumber or wood plank decking secured with smaller fasteners than 8d nails or with fewer than two nails (minimum size 8d) to each framing member it crosses shall be deemed sufficiently connected if fasteners are added such that two clipped head, round head, or ring shank nails (minimum size 8d) are in place on each framing member it crosses. 706.7.1.2 For roof decking consisting of wood structural panels, fasteners and spacing required in columns 3 and 4 of Table 706.7.1.2 are deemed to comply with the requirements of Section 707.3, Florida Building Code, Existing Building for the indicated design wind speed range. Wood structural panel connections retrofitted with a two part urethane based closed cell adhesive sprayed onto the joint between the sheathing and framing members are deemed to comply with the requirements of Section 707.3, Florida Building Code, Existing Building, provided testing using the manufacturer's recommended application on panels connected with 6d smooth shank nails at no more than a 6-inch edge and 12-inch field spacing demonstrate an uplift resistance of a minimum of 200 psf. Supplemental fasteners as required by Table 706. 7.1.2 shall be 8d ring shank nails with round heads and the following minimum dimensions: 1. 0.113-inch nominal shank diameter. 2. Ring diameter a minimum of 0.010 inch over shank diameter. 3. 16 to 20 rings per inch. 4. A minimum 0.280-inch full round head diameter. 5. Ring shank to extend a minimum of 1 1'2 inches from the tip of the nail. 6. Minimum 2 3/8-inch nail length. Secondary Water Barrier FBC - Existing Building Section 708.7 establishes the requirement that a secondary water barrier system be installed. NOTE: Structures that already comply are exempt from this requirement, (i.e. permitted under the 2007 code or newer). FBC - Existing Building Section 706.7.2 establishes the various methods of installing a system that complies with the secondary water barrier system requirement. 706.7.2 Roof secondary water barrier for site-built single family residential structures. A secondary water barrier shall be installed using one of the following methods when roof covering is removed and replaced

R905.1.1 Underlayment. Unless otherwise noted underlayment for asphalt shingles, metal roof shingles, mineral-surfaced roll roofing, slate and slate type shingles, wood shingles, wood shakes and metal roof panels shall conform to the applicable standards listed in this chapter. Underlayment materials required to comply with ASTM D226, D1970, D4869 and D6757 shall bear a label indicating compliance to the standard designation and, if applicable, type classification indicated in Table R905.1.1. Underlayment shall be applied and attached in accordance with Table R905.1.1. Exception: A reinforced synthetic underlayment that is approved as an alternate to underlayment complying with ASTM D226 Type II and having a minimum tear strength in accordance with ASTM D 1970 or ASTM D4533 of 20 pounds shall be permitted. This underlayment shall be installed and attached in accordance with the underlayment attachment methods of Table R905.1.1 for the applicable roof covering and slope, except metal cap nails shall be required where the ultimate design wind speed, V ult equals or exceeds 150 mph

A7.1 APPROVED FASTENERS FOR WOOD FRAME CONNECTIONS

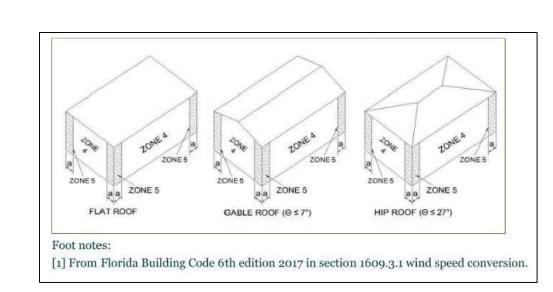
SIMPSON STRAPS, HANGERS, ANCHORS, PLATES LISTED PER CONNECTION AND SHOULD BE USED AT EVERY SIMILAR CONNECTION SEE AND FOLLOW MANF INSTALLATION INSTRUCTION FOR ALL FASTENERS SIZES LENGTHS AND QUANTITY

a pourie crups	(1)	
9. DOUBLE STUDS	16d (3½"X0.135") @ 24"0.C. 3"X0.131" NAIL @ 8"0.C. 3" 14 GAGE STAPLE @ 8"0.C.	FACE NAIL
10. DOUBLE TOP PLATES	16d (3½"X0.135") @ 16"0.C. 3"X0.131" NAIL @ 12"0.C. 3" 14 GAGE STAPLE @ 12"0.C.	TYPICAL FACE NAIL
-DOUBLE TOP PLATES	(16)16d COMMON (3½"X0.162") (24)3"X0.131" NAILS (24)3" 14 GAGE STAPLES	LAP SPLICE
11. BLOCKING BETWEEN JOISTS OR RAFTERS TO TOP PLATE	(3)8d COMMON (2½"X0.131") (3)3"X0.131" NAILS (3)3" 14 GAGE STAPLES	TOENAIL
12. RIM JOIST TO TOP PLATE	8d (2½"X0.131") @ 6"O.C. 3"X0.131" NAILS @ 6"O.C. 3" 14 GAGE STAPLES @ 6"O.C.	TOENAIL
13. TOP PLATES, LAPS, AND INTERSECTIONS	(2)16d COMMON (3½"X0.162") (3)3"X0.131" NAILS (3)3" 14 GAGE STAPLES	FACE NAIL
14. CONTINUOUS HEADER, TWO PIECES	(2)16d COMMON (3½"X0.162")	16"O.C. ALONG EDGE
15. CEILING JOISTS TO PLATE	(3)8d COMMON (2½"X0.131") (5)3"X0.131" NAILS (5)3" 14 GAGE STAPLES	TOENAIL
16. CONTINUOUS HEADER TO STUD	(4)8d COMMON (2½"X0.131")	TOENAIL
17. CEILING JOISTS, LAPS OVER PARTITIONS (SEE SECTION 2308.10.4.1, TABLE 2308.10.4.1)	(3)16d COMMON (3½"X0.162") MIN. TABLE 2308.10.4.1 (4)3"X0.131" NAILS (4)3" 14 GAGE STAPLES	FACE NAIL
18. CEILING JOISTS TO PARALLEL RAFTERS (SEE SECTION 2308.10.4.1, TABLE 2308.10.4.1)	(3)16d COMMON (3½"X0.162") MIN. TABLE 2308.10.4.1 (4)3"X0.131" NAILS (4)3" 14 GAGE STAPLES	FACE NAIL
19. RAFTER TO PLATE (SEE SECTION 2308.10.1, TABLE 2308.10.1)	(3)8d COMMON (2½"X0.131") (3)3"X0.131" NAILS (3)3" 14 GAGE STAPLES	TOENAIL
20. 1" DIAGONAL BRACE TO EA. STUD & PLATE	(2)8d COMMON (2½"X0.131") (2)3"X0.131" NAILS (3)3" 14 GAGE STAPLES	FACE NAIL
21. 1"X8" SHEATHING TO EA. BEARING	(3)8d COMMON (2½"X0.131")	FACE NAIL
22. WIDER THAN 1"X8" SHEATHING TO EA. BEARING	(3)8d COMMON (2½"X0.131")	FACE NAIL
23. BUILT-UP CORNER STUDS	16d COMMON (3½"X0.162") 3"X0.131" NAILS 3" 14 GAGE STAPLES	24"0.C. 16"0.C. 16"0.C.
24. BUILT-UP GIRDER AND BEAMS	20d COMMON (4"X0.192") 32"O.C. 3"X0.131" NAILS @ 24"O.C. 3" 14 GAGE STAPLES @ 24"O.C.	FACE NAIL @ TOP AND BOTTOM STAGGERED ON OPPOSITE SIDES
	(2)20d COMMON (4"X0.192") (3)3"X0.131" NAILS (3)3" 14 GAGE STAPLES	FACE NAIL @ ENDS AND @ EA. SPLICE
25. 2" PLANKS	16d COMMON (3½"X0.162")	@ EACH BEARING
26. COLLAR TIE TO RAFTER	(3)10d COMMON (3"X0.148") (4)3"X0.131" NAILS (4)3" 14 GAGE STAPLES	FACE NAIL
27. JACK RAFTER TO HIP	(3)10d COMMON (3"X0.148") (4)3"X0.131" NAILS (4)3" 14 GAGE STAPLES	TOENAIL
	(2)16d COMMON (3½"X0.162") (3)3"X0.131" NAILS (3)3" 14 GAGE STAPLES	FACE NAIL
28. ROOF RAFTER TO 2-BY RIDGE BEAM	(2)16d COMMON (3½"X0.162") (3)3"X0.131" NAILS (3)3" 14 GAGE STAPLES	TOENAIL

2020 FBC 107.3.5 Exterior wall envelope assemblies shall be tested at a minimum differential pressure of 6.24 pounds per square foot (psf) (0.297 kN/m 2). 2.4. Exterior wall envelope assemblies shall be subjected to a minimum test exposure duration of 2 hours.

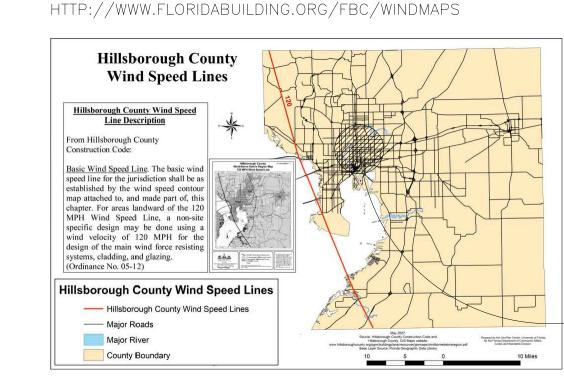
<i>Project:</i> 103 Prepared by: E-mail:			s LLC						
Input: ASCE 7-10 S Wind Speed Building Exp Internal Pres Roof angle is Kzt = 1.00 Kd = 0.85 Mean Roof I	= 120 mp oosure = E ssure Coef s less than	h 3 (Urban & fficient GCp or equal to	Suburban a oi = 0.18 Er	areas) iclosed Bu	ilding	Coefficient	is reduced	10%)	
Output: Velocity Pres			icient "Kz r	max." = 0.	7				
Per Table 30.5 LRFD uses w Velocity Pres Per eq.30.3-1, ASD uses wi Velocity Pres	vind speed ssure (qh l page 316 nd speed l ssure (qh l	Vult = 120 LRFD) = 21 Vasd ¹ = 93 ASD) = 13.1	mph 16 psf = .00	0256*Kz*k	Zt*Kd*V^:	2*0.6 lb/ft	^2	gure 30.4	-1, Page
LRFD uses v Velocity Pres Per eq.30.3-1, ASD uses wi Velocity Pres	vind speed ssure (qh l page 316 nd speed l ssure (qh l LRFD Fa	Vult = 120 LRFD) = 21 Vasd ¹ = 93 ASD) = 13.1 (Load Res	mph 16 psf = .00 sistance gn)	ASD (A	Zzt*Kd*V^: Allowable Design)	2*0.6 lb/ft e Stress	^2 (Per Fi	335) ernal Pres	ssure
LRFD uses v Velocity Pres Per eq.30.3-1, ASD uses wi Velocity Pres	vind speed ssure (qh l page 316 nd speed l ssure (qh l LRFD Fa	Vult = 120 LRFD) = 21 Vasd ¹ = 93 ASD) = 13.1 (Load Res	mph 16 psf = .00	ASD (A	Zzt*Kd*V^: Allowable Design)	2*0.6 lb/ft e Stress	^2 (Per Fi	335) ernal Pres coefficien Negative Center	ssure ts Negativ
LRFD uses v Velocity Pres Per eq.30.3-1, ASD uses wi Velocity Pres — — Tributary Area	vind speed ssure (qh l page 316 nd speed v ssure (qh l LRFD (Fa Positive Zone 4 & 5	Vult = 120 LRFD) = 21 Vasd ¹ = 93 ASD) = 13.1 (Load Resector Designative Center Zone 4	mph 16 psf = .00 sistance gn) Negative Corner Zone 5	ASD (ASD (ASD (ASD (ASD (ASD (ASD (ASD (Allowable Design) Negative Center Zone 4	2*0.6 lb/ft 2 Stress Negative Corner Zone 5	(Per Fi	335) ernal Pres Coefficien Negative	ssure ts Negati Corne GCp
LRFD uses v Velocity Pres Per eq.30.3-1, ASD uses wi Velocity Pres — Tributary Area Sq Feet	LRFD (Fa Positive Zone 4 & 5 psf	Vult = 120 LRFD) = 21 Vasd¹ = 93 ASD) = 13.1 (Load Resctor Desi Negative Center Zone 4 psf	mph 16 psf = .00 sistance gn) Negative Corner Zone 5 psf	ASD (ASD (ASD (ASD (ASD (ASD (ASD (ASD (Allowable Design) Negative Center Zone 4 psf	2*0.6 lb/ft e Stress Negative Corner Zone 5 psf	(Per Fi	335) ernal Pres Coefficien Negative Center GCp	ssure ts Negati Corne GCp -1.26
LRFD uses v Velocity Pres Per eq.30.3-1, ASD uses wi Velocity Pres — Tributary Area Sq Feet 10 ft²	vind speed sure (qh) page 316 nd speed v ssure (qh) LRFD (Fa Positive Zone 4 & 5 psf 23.7	Vult = 120 LRFD) = 21 Vasd¹ = 93 ASD) = 13.1 (Load Resctor Designative Center Zone 4 psf	mph 16 psf = .00 sistance gn) Negative Corner Zone 5 psf -31.6	ASD (ASD (ASD (ASD (ASD (ASD (ASD (ASD (Allowable Design) Negative Center Zone 4 psf -16	2*0.6 lb/ft Stress Negative Corner Zone 5 psf -18.9	Positive GCp	335) ernal Pres Coefficien Negative Center GCp -0.99	Negati Corne GCp -1.26
LRFD uses v Velocity Pres Per eq.30.3-1, ASD uses wi Velocity Pres Tributary Area Sq Feet 10 ft ² 20 ft ²	vind speed ssure (qh) page 316 nd speed ssure (qh) LRFD (Fa Positive Zone 4 & 5 psf 23.7 22.6	Vult = 120 LRFD) = 21 Vasd¹ = 93 ASD) = 13.1 (Load Resctor Desi Negative Center Zone 4 psf -25.7 -24.6	mph 16 psf = .00 sistance gn) Negative Corner Zone 5 psf -31.6 -29.5	ASD (ASD (ASD (ASD (ASD (ASD (ASD (ASD (Allowable Design) Negative Center Zone 4 psf -16 -16	2*0.6 lb/ft Stress Negative Corner Zone 5 psf -18.9 -17.7	Positive GCp	335) ernal Pres Coefficien Negative Center GCp -0.99	ssure ts
LRFD uses w Velocity Pres Per eq.30.3-1, ASD uses wi Velocity Pres Tributary Area Sq Feet 10 ft ² 20 ft ² 50 ft ²	vind speed ssure (qh) page 316 nd speed ssure (qh) LRFD (Fa Positive Zone 4 & 5 psf 23.7 22.6 21.2	Vult = 120 LRFD) = 21 Vasd ¹ = 93 ASD) = 13.1 (Load Resctor Desi Negative Center Zone 4 psf -25.7 -24.6 -23.2	mph 16 psf = .00 sistance gn) Negative Corner Zone 5 psf -31.6 -29.5 -26.7	ASD (ASD (ASD (ASD (ASD (ASD (ASD (ASD (Allowable Design) Negative Center Zone 4 psf -16 -16	2*0.6 lb/ft Stress Negative Corner Zone 5 psf -18.9 -17.7 -16.0	^2 (Per Fi Exte (C) Positive GCp 0.90 0.85 0.79	335) crnal Prescoefficien Negative Center GCp -0.99 -0.94	Negative Corne GCp -1.26 -1.16

ASCE 7-10 Wind load Program (30.4 Part 1: Low-Rise Building Wall Components and Cladding)



RISK CATEGORY II

CLIMATE ZONE 2A FBC 1609 Wind-loads ASCE 7-10 to ASCE 7-16



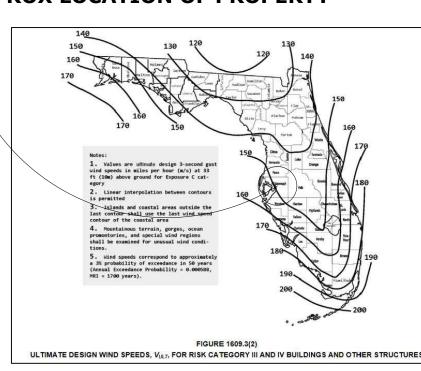
HILLSBOROUGH COUNTY GIS MAP INDICATES RISK CAT II HVHZ WIND

ZONE 120 MPH WIND BORNE DEBRIS REGION AND BASIC WIND SPEED MAP. THE COUNTY WIND BORNE DEBRIS REGION AND BASIC WIND SPEED MAP ESTABLISHES THE GEOGRAPHICAL BOUNDARIES OF THE WIND SPEED ZONES AND THE WIND BORNE DEBRIS REGION IN THE COUNTY.

EXPOSURE CATEGORY B

Exposure category, Exposure B. For buildings with a mean roof height of less than or equal to 30 feet (9144 mm), Exposure B shall apply where the ground surface roughness, as defined by Surface Roughness B, prevails in the upwind direction for a distance of not less than 1,500 feet (457 m). Surface Roughness B. Urban and suburban areas, wooded areas or other terrain with numerous closely spaced obstructions having the size of single-family dwellings or larger.

APPROX LOCATION OF PROPERTY



SEE NOTES ON LEFT FOR ROOF AND UNDERLAYMENT ATTACHMENT DETAILS

SEE A2.1 c. Swing Door

APPROVED

SEE A2.1 d. Windows By Benjamin Daniels at 3/10/2023 8:11:04 AM

Southern Pine Span Chart Visually graded #2 2x12 RIM JOIST BEAMS

DECK BEAM SPANS

TABLE R507.6

	BEAM SPAN LENGTHS ^a									
JOIST SPAN	BEAM SIZE									
JUIST SPAN	(2)2x6	(2)2x8	(2)2x10	(2)2x12	(3)2x6	(3)2x8	(3)2x10	(3)2x12		
≤ 6'	7'-1"	9'-2"	11'-10"	13'-11"	8'-7"	11'-4"	14'-5"	17'-5"		
6' - 8'	6'-2"	7'-11"	10'-3"	12'-0"	7'-8"	9'-11"	12'-10"	15'-1"		
8' - 10'	5'-6"	7'-1"	9'-2"	10'-9"	6'-11"	8'-11"	11'-6"	13'-6"		
10' - 12'	5'-0"	6'-6"	8'-5"	9'-10"	6'-3"	8'-1"	10'-6"	12'-4"		
12' - 14'	4'-8"	6'-0"	7'-9"	9'-1"	5'-10"	7'-6"	9'-9"	11'-5"		
14' - 16'	4'-4"	5'-7"	7'-3"	8'-6"	5'-5"	7'-0"	9'-1"	10'-8"		
16' - 18'	4'-1"	5'-3"	6'-10"	8'-0"	5'-2"	6'-7"	8'-7"	10'-1"		

For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm a. Tabulated values are based on southern pine, grade #2, wet service.

.6.1 Beam bearing. Beam bearing shall be provided at posts in accordance with Section R502.6 and Figure R507.6.1. Post if used, shall have a minimum capacity of 5,000 pounds (22.25 kN) and shall be specifically manufactured for the beam and

CEILING FRAMING

DEAD LOAD AND LIVE LOADS FOR PROPOSED DEAD LOAD MIN FOR RESIDENTIAL 10SPF LIVE LOAD MIN 40PSF 360 DEFLECTION

TABLE R301.5

USE	LIVE LOAD	
Uninhabitable attics without storage ^b	10	
Uninhabitable attics with limited storage ^{b, g}	20	
Habitable attics and attics served with fixed stairs	30	
Balconies (exterior) and decks ^e	40	
Fire escapes	40	
Guards and handrails ^d	200 ^h	
Guard in-fill components ^f	50 ^h	
Passenger vehicle garages ^a	50 ^a	
Rooms other than sleeping rooms	40	
Sleeping rooms	30	
Stairs	40 ^c	
	2 4	

R301.2.1.2 Protection of Openings

Exterior glazed openings in buildings located in <u>windborne</u> debris regions shall be protected from windborne debris. Glazed opening protection for windborne debris shall meet the requirements of the Large Missile Test of ASTM E1996 and ASTM E1886 as modified in Section 301.2.1.2.1, TAS 201, 202 and 203, or AAMA 506, as applicable. Garage door glazed opening protection for windborne debris shall meet the requirements of an <u>approved</u> impact—resisting standard or ANSI/DASMA 115.

1. Attachments shall be designed to resist the component and <u>cladding</u> loads determined in accordance with either <u>Table R301.2(2)</u> or ASCE 7, with the permanent corrosion-resistant attachment hardware provided and anchors permanently installed on the building.

PLAN APPROVAL

Pare BUILT Architecture Building - Benja Urban Design - Andy Mikulski - Approved with Comme Natural Resources - Michael Lousias - Approved with

OWNER

ADAM DUFF

Comments - 3/20/

ADDRESS

105 WEST HILDA AVE TAMPA, FL 33603

DRAWINGS BY

LESLIE SIMPSON 404 414 7850 lsimpson@lgsarchdesign.com

CONTRACTOR

ANDRES PENA 813 470 8323 apena9@me.com

ENGINEER

N/A

DATE:

DECEMBER 2022

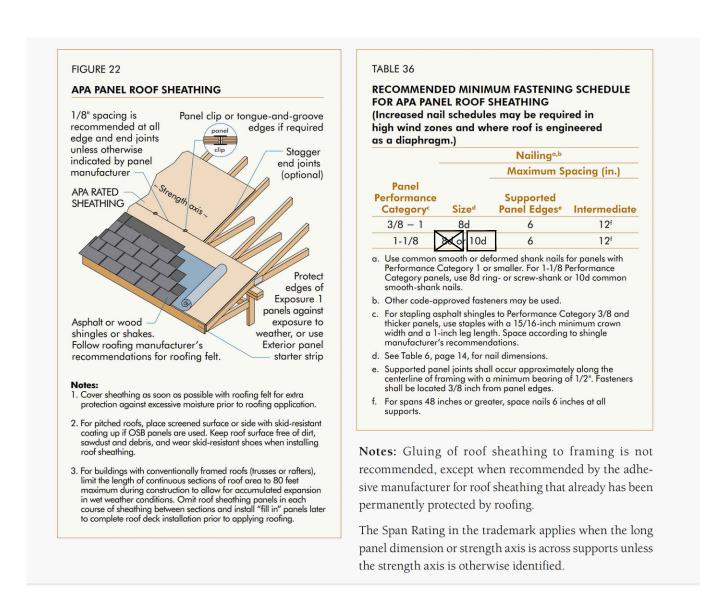
PROJECT NAME

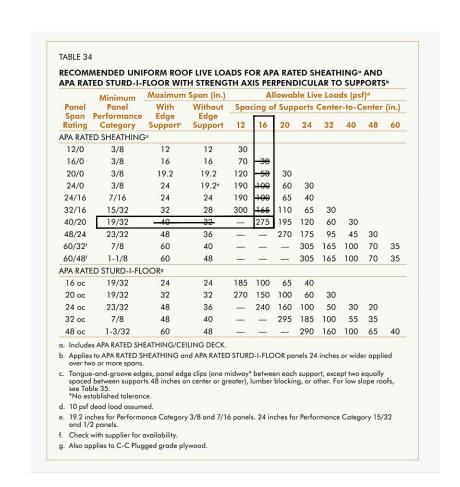
NEW HOME

105 W HILDA

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Sheet





INSTALLATION OF LATH & STUCCO PER **ASTM C-926 - 11a** ASTM C-1063 -03

NOMINAL MATERIA	TUICKNESS	DESCRIPTION ^{a, b} OF FASTENER AND LENGTH			SPACING ^c OF FASTENERS		
(inches		DESC	(inches)	Edges (inches)	Intermediate suppo (inches)		
	Wood structural pane	ls subfloor, roof ^g	and wall sheathing to framing and particleboard wa	II sheathing to fran	ning ^f		
			Staple 15 ga. 1 ³ / ₄	4	8		
Up to 1/	2		0.097 - 0.099 Nail 2 ¹ / ₄	3	6		
		Staple 16 ga. 1 ³ / ₄			6		
		0.113 Nail 2		3	6		
¹⁹ / ₃₂ and	5/8	Staple 15 and 16 ga. 2			8		
		0.097 - 0.099 Nail 2 ¹ / ₄			8		
	$\neg \neg \vdash$		Staple 14 ga. 2	4	8		
²³ / ₃₂ and	3,	Staple 15 ga. 1 ³ / ₄		3	6		
/ ₃₂ and	14	0.097 - 0.099 Nail 2 ¹ / ₄		4	8		
			Staple 16 ga. 2		8		
		Staple 14 ga. 2 ¹ / ₄			8		
4			0.113 Nail 2 ¹ / ₄	3	6		
1			Staple 15 ga. 2 ¹ / ₄		8		
			0.097 - 0.099 Nail 2 ¹ / ₂	4	8		

103 WEST HILDA AVE

ROOF DECKING TO ROOF BOARD CONNECTION REQUIREMENTS TO FRAMING

TABLE 2 – ZONE 5 SIDING ATTACHMENT REQUIREMENTS FOR SUCTION BASED ON THE 2006 AND 2009 INTERNATIONAL RESIDENTIAL CODES (PSF) (BASED ON A MEAN ROOF HEIGHT OF 30 FEET AND LOCATED IN

Wind Speed (mph)	85	90	100	105	110	120	125	130	140	145	150	170
Withdrawal Requirement (psf)	17.4	19.5	24.1	26.6	29.1	34.7	37.6	40.7	47.2	50.6	54.2	69.6

(a) An effective wind area of 10 sq. ft. was used in accordance with Footnote (a) to Table R301.2(2) of the 2006 and 2009 IRC.

(b) For other mean roof heights and wind exposures, values in the above table above shall be multiplied by the following adjustment factors:

Manus Band Haimba (ff)		Exposure	
Mean Roof Height (ft)	В	С	D
15	1.00	1.21	1.47
20	1.00	1.29	1.55
25	1.00	1.35	1.61
30	1.00	1.40	1.66
35	1.05	1.45	1.70
40	1.09	1.49	1.75
45	1.12	1.53	1.78
50	1.16	1.56	1.81
55	1.19	1.59	1.84
60	1.22	1.62	1.87

TABLE 1A - RING-SHANK NAIL WITHDRAWAL LOADS (POUNDS)(c)

Plywood or OSB Performance	Ring-Shank Nails ^(b,e) , Diameter (in.)									
Category	0.091	0.094	0.097	0.113	0.120	0.128	0.135	0.148		
3/8	31	32	33	38	41	43	46	50		
7/16	36	37	38	45	48	51	53	59		
15/32 and 1/2	39	40	41	48	51	54	57	63		
19/32 and 5/8	49	51	52	/ js/ /	64	69	73	80		
23/32 and 3/4	59	61	63	74	78	83	88	96		

TABLE 1B - WOOD SCREW WITHDRAWAL LOADS (POUNDS)(a)

Plywood or OSB	Wood Screws ^(c,e) , Gauge/Diameter (in.)								
Performance	#6	#7	#8	#9	#10	#12	#14		
Category	0.138	0.151	0.164	0.177	0.190	0.216	0.242		
3/8	48	52	57	61	66	75	84		
7/16	56	61	66	72	77	87	98		
15/32 and 1/2	60	65	71	77	82	93	105		
19/32 and 5/8	76	83	90	97	104	118	133		
23/32 and 3/4	92	100	109	117	126	143	161		

Design Uplift Pressures:

Table "A" Maximum Design Pressures						
Roof Areas	Assembly A	Assembly B	Assembly C			
Maximum Design Pressures	-63.5 psf	-121.75 psf	-161 psf			
Fastener Spacing	5%" O.C	51/8" O.C	51/8" O.C			
Sealant	No	Yes	No			
Panel Clip	No	No	Yes			

 $\frac{19}{32}$ AND $\frac{5}{8}$ PLYWOOD ROOF DECKING CONNECTION REQUIREMENTS TO FRAMING 61LB IS THE MIN ACCORDING TO IRC CALCULATIONS FOR EXPOSURE RATING

4x8 SHEATHING NAIL PATTERN TYP

•	0	0	0	0	0	•
	N.	AIL PATTE	RN TYP			
•	• 6"	OC SUPI	PORTED	PANEL E	DGE •	9
•	12	OC INT	FRIOR PA	ANFI TYF)	
•	0	0	0	0	0	•
—	o	o	0	o	0	•
•						•
•	•	0	0	0	0	•
14	П					

FASTENING SPECIFICATIONS

1506.5Nails.

Nails shall be corrosion-resistant nails conforming to ASTM F1667 or an equal corrosion resistance by coating, electro galvanization, mechanical galvanization, hotdipped galvanization, stainless steel, nonferrous metal and alloys or other suitable corrosion-resistant material, or corrosion resistance shall be demonstrated in accordance with TAS114, Appendix E.

1506.6Screws.

Wood screws conform to ANSI/ASME B18.6.1. Screws shall be corrosion resistant by coating, galvanization, stainless steel, nonferrous metal or other suitable corrosion-resistant material. The corrosion resistance shall be demonstrated through one of the following methods:

- 1. Corrosion resistance equivalent to ASTM A641, Class 1;
- 2.Corrosion resistance in accordance with TAS114, Appendix E; or
- 3. Corrosion-resistant coating exhibiting not more than 5 percent red rust after 1000 hours exposure in accordance with ASTM B117.

1506.7Clips.

Clips shall be corrosion-resistant clips. The corrosion resistance shall meet 0.90 ounce per square foot (0.458 kg/m2) measured according to ASTM A90/A90M, TAS 114, Appendix E or an equal corrosion-resistance coating, electro galvanization, mechanical galvanization, hot dipped galvanization, stainless steel, nonferrous metals and alloys or other suitable corrosion-resistant material. Stainless steel clips shall conform to ASTM A240/A240M, Type 304.

1507.1.1Underlayment.

Underlayment for roof slopes 2:12 and greater shall conform to the applicable standards listed in this chapter. Underlayment materials required to comply with ASTM D226, D1970, D4869 and D6757 shall bear a label indicating compliance to the standard designation and, if applicable, type classification indicated. Underlayment for roof slopes 2:12 and greater shall be applied and attached in accordance with Section 1507.1.1.1, 1507.1.1.2 or 1507.1.1.3 as applicable.

Exceptions:

- 1. For areas of a roof that cover exterior walkways and roofs of agricultural buildings, underlayment shall comply with the manufacturer's installation instructions.
- 2.Compliance with Section 1507.1.1.1 is not required for structural metal panels that do not require a substrate or underlayment.
- 1507.1.1.1Underlayment for asphalt, metal, mineral surfaced, slate and slate-type roof coverings. Underlayment for asphalt shingles, metal roof shingles, mineral surfaced roll roofing, slate and slate-type shingles, and metal roof panels shall comply with one of the following methods:
- 1.The entire roof deck shall be covered with an approved self-adhering polymer modified bitumen underlayment complying with ASTM D1970 installed in accordance with both the underlayment manufacturer's and roof covering manufacturer's installation instructions for the deck material, roof ventilation configuration and climate exposure for the roof covering to be installed.

Exception: An existing self-adhering modified bitumen underlayment that has been previously installed over the roof decking and, where it is required, renailing off the roof sheathing in accordance with Section 706.7.1 of the Florida Building Code, Existing Building can be confirmed or verified. An approved underlayment in accordance with Table 1507.1.1.1 for the applicable roof covering shall be applied over the entire roof over the existing self-adhered modified bitumen underlayment.

2.A minimum 4-inch-wide (102 mm) strip of selfadhering polymer-modified bitumen membrane complying with ASTM D1970, installed in accordance with the manufacturer's instructions for the deck material, shall be applied over all joints in the roof decking. An approved underlayment in accordance with Table 1507.1.1.1 for the applicable roof covering shall be applied over the entire roof over the 4-inch-wide (102 mm) membrane strips.

1507.3.6Fasteners.

Tile fasteners shall be corrosion resistant and not less than 11-gage, 5/16-inch (8.0 mm) head, and of sufficient length to penetrate the deck a minimum of 3/4 inch (19.1 mm) or through the thickness of the deck, whichever is less. Attaching wire for clay or concrete tile shall not be smaller than 0.083 inch (2.1 mm). Perimeter fastening areas include three tile courses but not less than 36 inches (914 mm) from either side of hips or ridges and edges of eaves and gable rakes.

1507.2.6Fasteners.

Fasteners for asphalt shingles shall be galvanized, stainless steel, aluminum or copper roofing nails, minimum 12-gage [0.105 inch (2.67 mm)] shank with a minimum 3/8-inch-diameter (9.5 mm) head, of a length to penetrate through the roofing materials and a minimum of 3/4 inch (19.1 mm) into the roof sheathing. Where the roof sheathing is less than 3/4 inch (19.1 mm) thick, the nails shall penetrate through the sheathing. Fasteners shall comply with ASTM F1667.

1507.2.6.1

The nail component of plastic cap nails shall meet the corrosion-resistance requirements of Section

1507.2.7Attachment.

Asphalt shingles shall have the minimum number of fasteners required by the manufacturer and Section 1504.1. Asphalt shingles shall be secured to the roof with not less than four fasteners per strip shingle or two fasteners per individual shingle. Where the roof slope exceeds 21 units vertical in 12 units horizontal (21:12), asphalt shingles shall be installed in accordance with the manufacturer's printed installation instructions for steep-slope roof applications.

1507.2.7.1 Wind resistance of asphalt shingles.

Asphalt shingles shall be classified in accordance with ASTM D3161, ASTM D7158 or TAS 107. Shingles classified as ASTM D3161 Class D or ASTM D7158 Class G are acceptable for use where Vasd is equal to or less than 100 mph. Shingles classified as ASTM D3161 Class F, ASTM D7158 Class H or TAS 107 are acceptable for use for all wind speeds. Asphalt shingle wrappers shall be labeled to indicate compliance with one of the required classifications, as shown in Table 1507.2.7.1.

> TAMPA FL 33603

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ADDRESS

105 WEST HILDA AVE TAMPA, FL 33603

DRAWINGS BY

LESLIE SIMPSON 404 414 7850 lsimpson@lgsarchdesign.com

CONTRACTOR

ANDRES PENA 813 470 8323 apena9@me.com

ENGINEER

N/A

DATE:

DECEMBER 2022

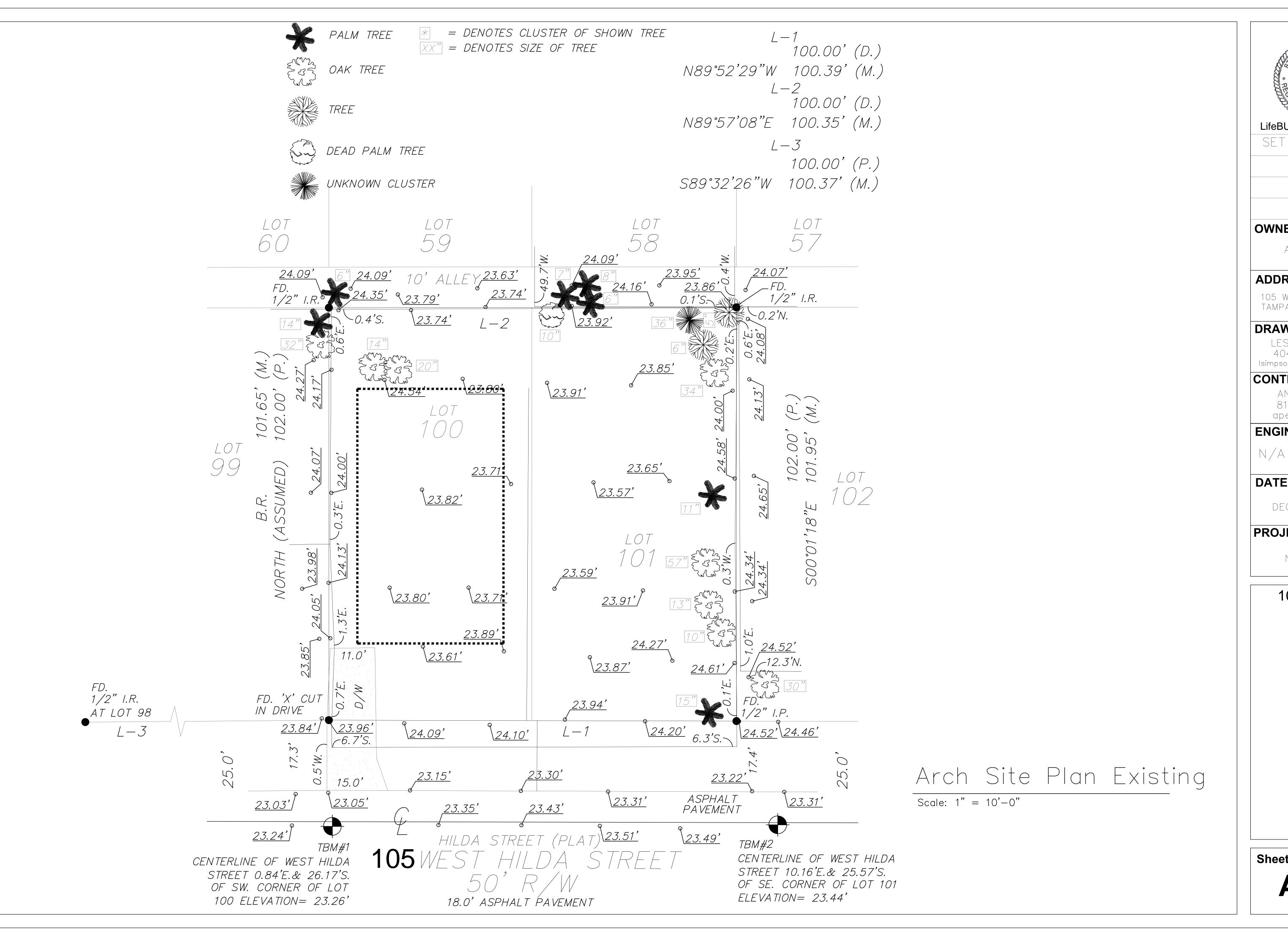
PROJECT NAME

NEW HOME

105 W HILDA

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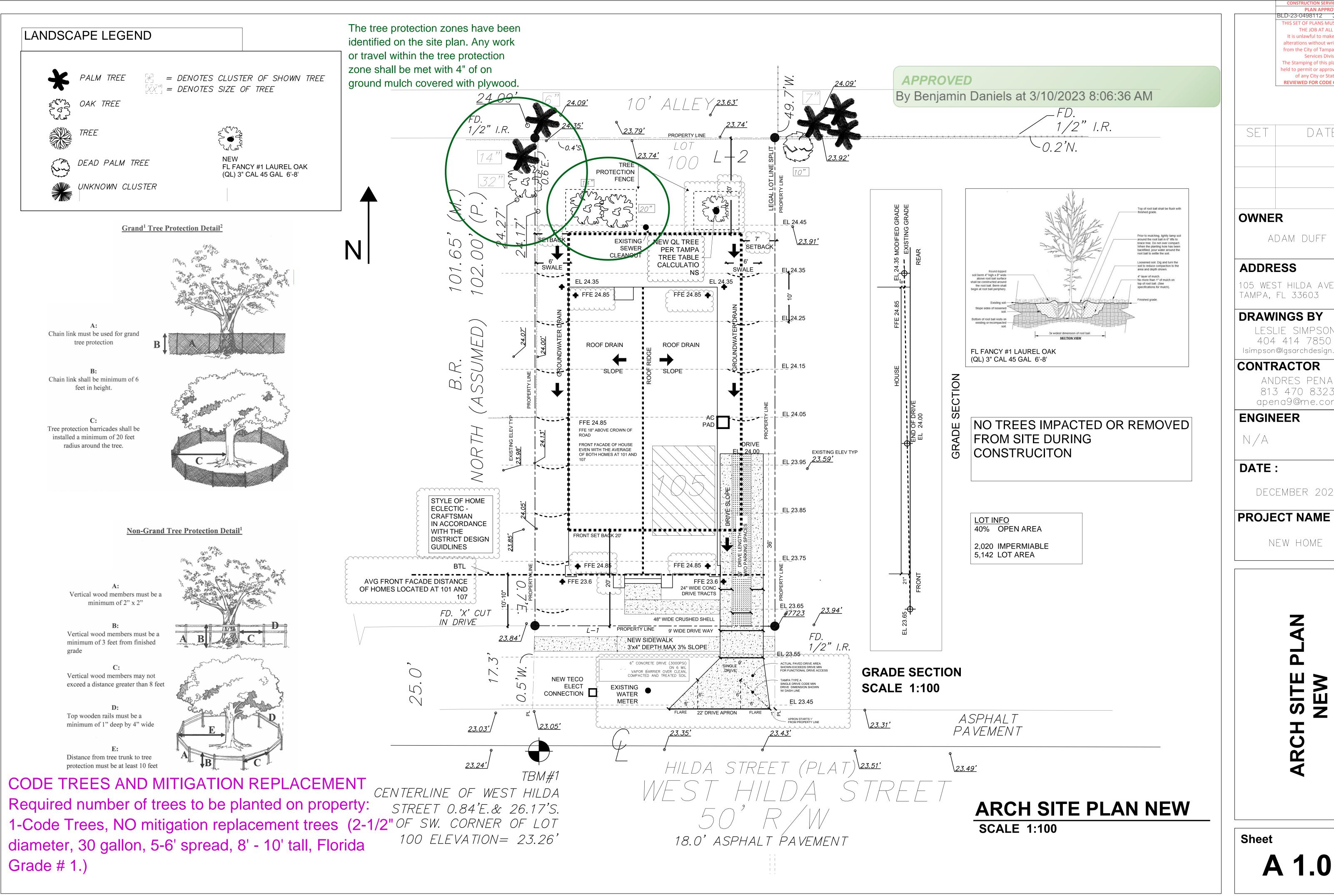
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PLAN APPROVAL THE JOB AT ALL TIMES It is unlawful to make change alterations without written app from the City of Tampa Cons The Stamping of this plan shall no neld to permit or approve the vio of any City or State Codes REVIEWED FOR CODE COMPLIANCE

DATE

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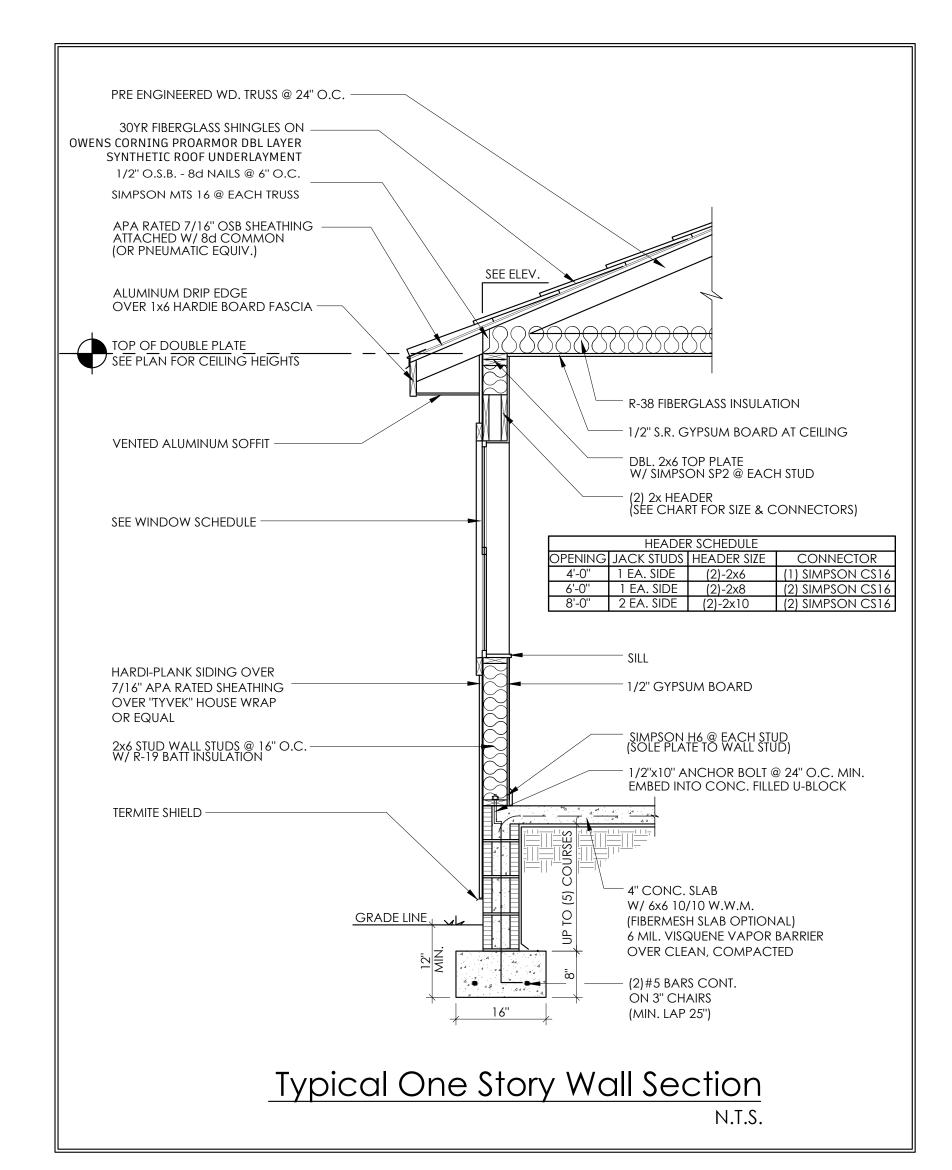
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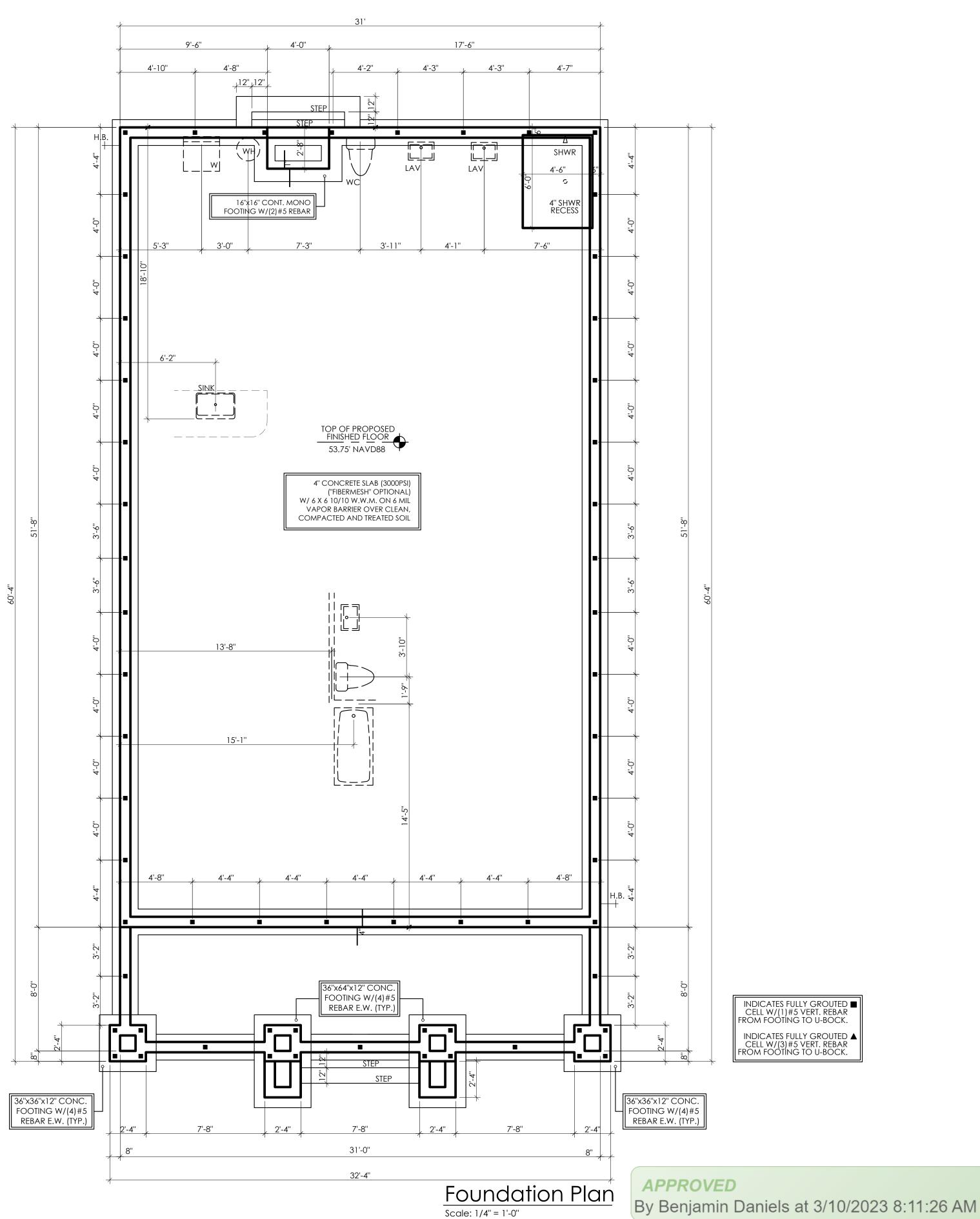
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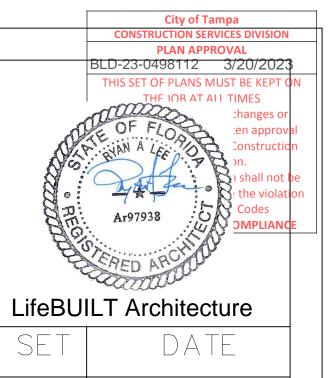
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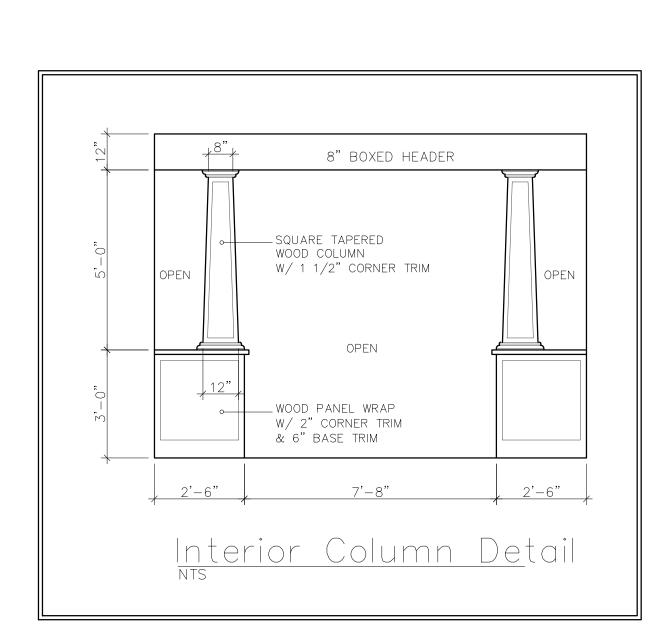
=OUNDATION PLAN

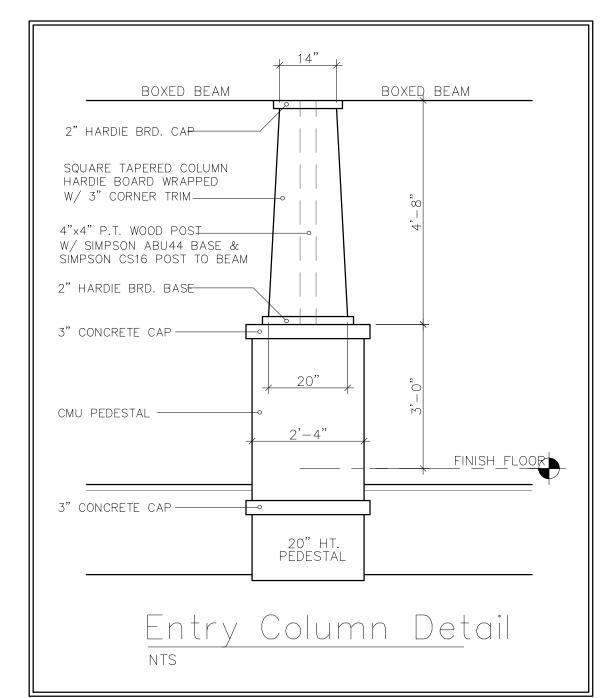
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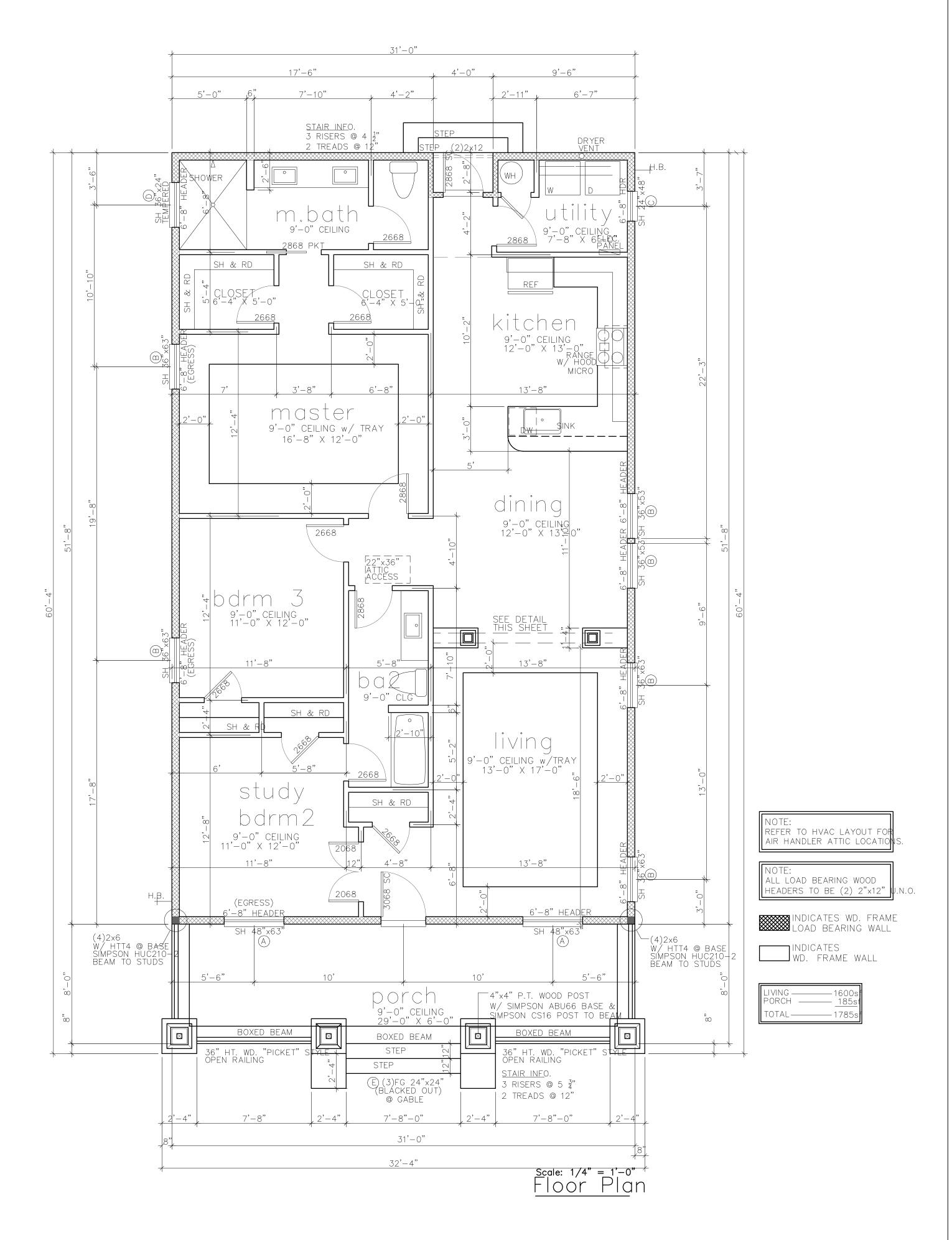
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Florida Product Approval Table				
Product Catagory	Product Catagory Sub Catagory		State Of Florida Approval Number	Approval Date
ROOFING	FIBERGLASS SHINGLE	CERTAINTEED	FL 10124	2020
ROOFING	UNDERLAYMENT	OWENS CORNING	FL 15216 R8	2020
SIDING	HARDI SIDING	JAMES HARDI	FL 13192	2020
WINDOW	SINGLE HUNG, HORIZ. SLIDER FIXED GLASS	SILVER LINE	FL 14911.5	2020
EXTERIOR DOOR	INSULATED FIBERGLASS	MASONITE	FL 29847.3	2020

	Window Schedule				
Mark	Size	Туре	Material	Glass Type	Color
A	SH 48"x63"	SINGLE HUNG	VINYL	DBL. GLAZED LOW-	E WHITE
B	SH 36"x63"	SINGLE HUNG	VINYL	DBL. GLAZED LOW-	E WHITE
(C)	SH 24"x48"	SINGLE HUNG	VINYL	DBL. GLAZED LOW-	E WHITE
(D)	FG 36"x24"	FIXED GLASS TEMPERE	D VINYL	DBL. GLAZED LOW-	E WHITE
E	FG 24"x24"	(BLACKED OUT)	VINYL	DBL. GLAZED LOW-	E WHITE







City of Tampa
CONSTRUCTION SERVICES DIVISION
PLAN APPROVAL
BLD-23-0498112 3/20/2023
THIS SET OF PLANS MUST BE KEPT ON
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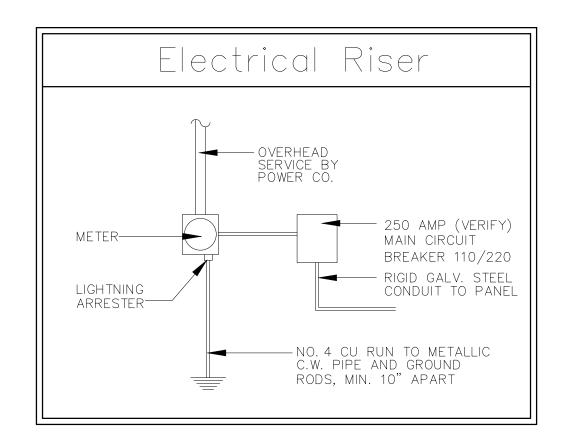
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OOR PLAN

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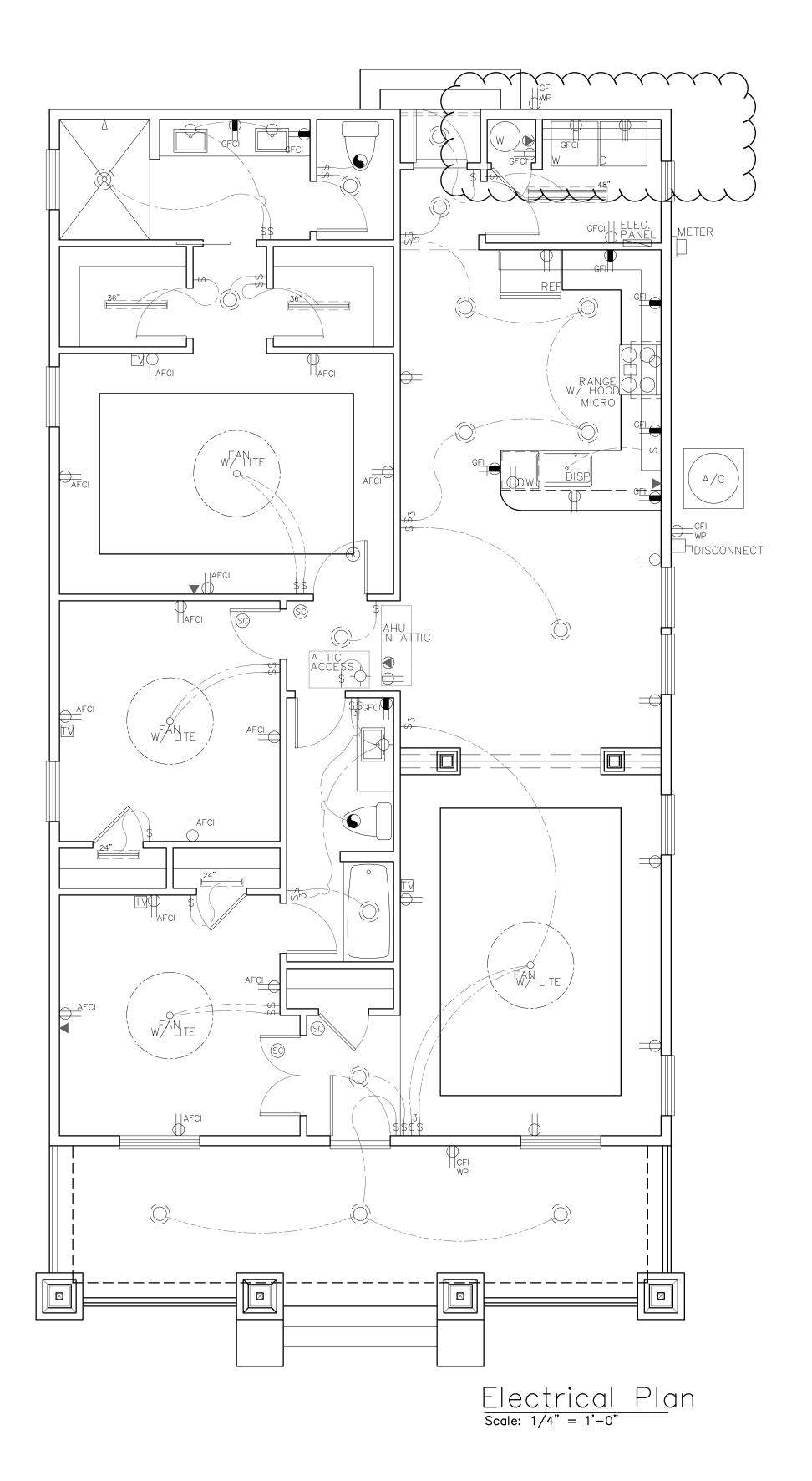
Electr	ical Symbols
\$	
\$3	SINGLE POLE SWITCH THREE (3) POLE SWITCH
\$ _D	REOSTAT SWITCH
\leftarrow	120v DUPLEX OUTLET
	120v DUPLEX OUTLET ONE OUTLET ON SWITCH
-	120v DUPLEX OUTLET COUNTER HEIGHT
₩P	DUPLEX OUTLET WEATHER PROOF GROUND FAUL
FLOOR	120v DUPLEX FLOOR OUTLET
	220v SERVICE OUTLET
	DIRECT WIRED
	DISCONNECT
-	CEILING LIGHT FIXTURE
+	WALL MOUNTED LIGHT FIXTURE
DECOR	DECORATIVE LIGHT FIXTURE OWNER SELECT
	RECESSED CEILING NON-INCANDESCENT LIGHT FIXTURE
OM	MICRO RECESSED CEILING LIGHT FIXTURE
(a)	UNDER COUNTER LED PUCK
ð	MAKEUP LIGHTING FIXTURE
	FLUORESCENT LIGHT FIXTURE
\triangle	DOUBLE FLOOD LIGHT
6	CEILING EXHAUST FAN
S	SMOKE DETECTOR
	CARBON MONOXIDE DETECTOR
TY	CABLE OUTLET
A	TELEPHONE JACK
C	CAT-6 NETWORK CABLE

Please Note:

- all circuits are to be either arcfault or gfci protected.

- all receptacles are to be child protected.

- smoke detectors are required inside and out of all sleeping sleeping areas, and carbon detectors are required within 10 of all sleeping rooms.



City of Tampa
CONSTRUCTION SERVICES DIVISION
PLAN APPROVAL
BLD-23-0498112 3/20/2023
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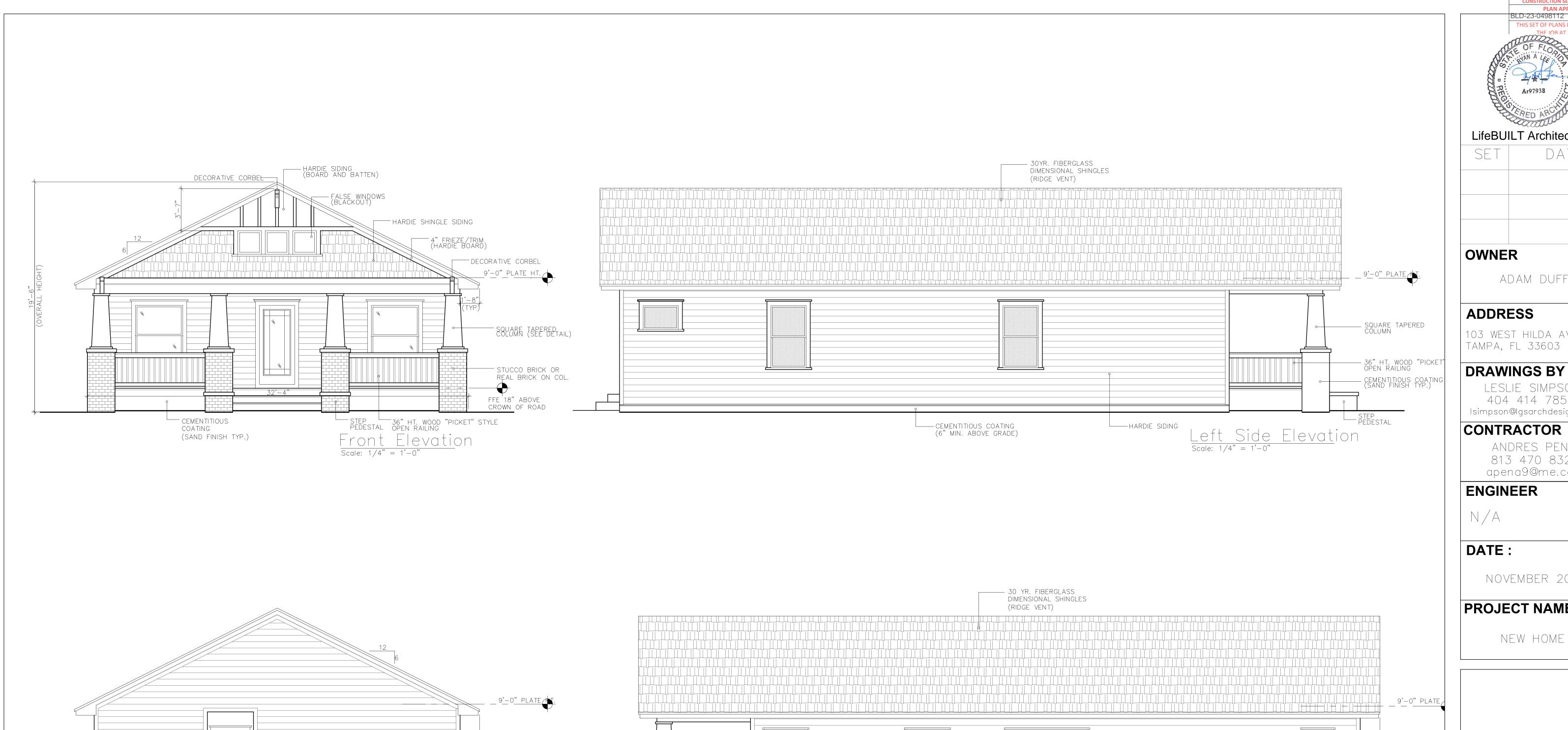
PROJECT NAME

NEW HOME

LECTRICAL PLAN

Sheet

A 2.1a



SQUARE TAPERED COLUMN

36" HT. WOOD "PICKET" STYLE OPEN RAILING

L—STEP PEDESTAL

CEMENTITIOUS COATING— (SAND FINISH TYP.)

----HARDIE SIDING

Rear Elevation

Scale: 1/4" = 1'-0"

— CEMENTITIOUS COATING (6" MIN. ABOVE GRADE)



City of Tampa

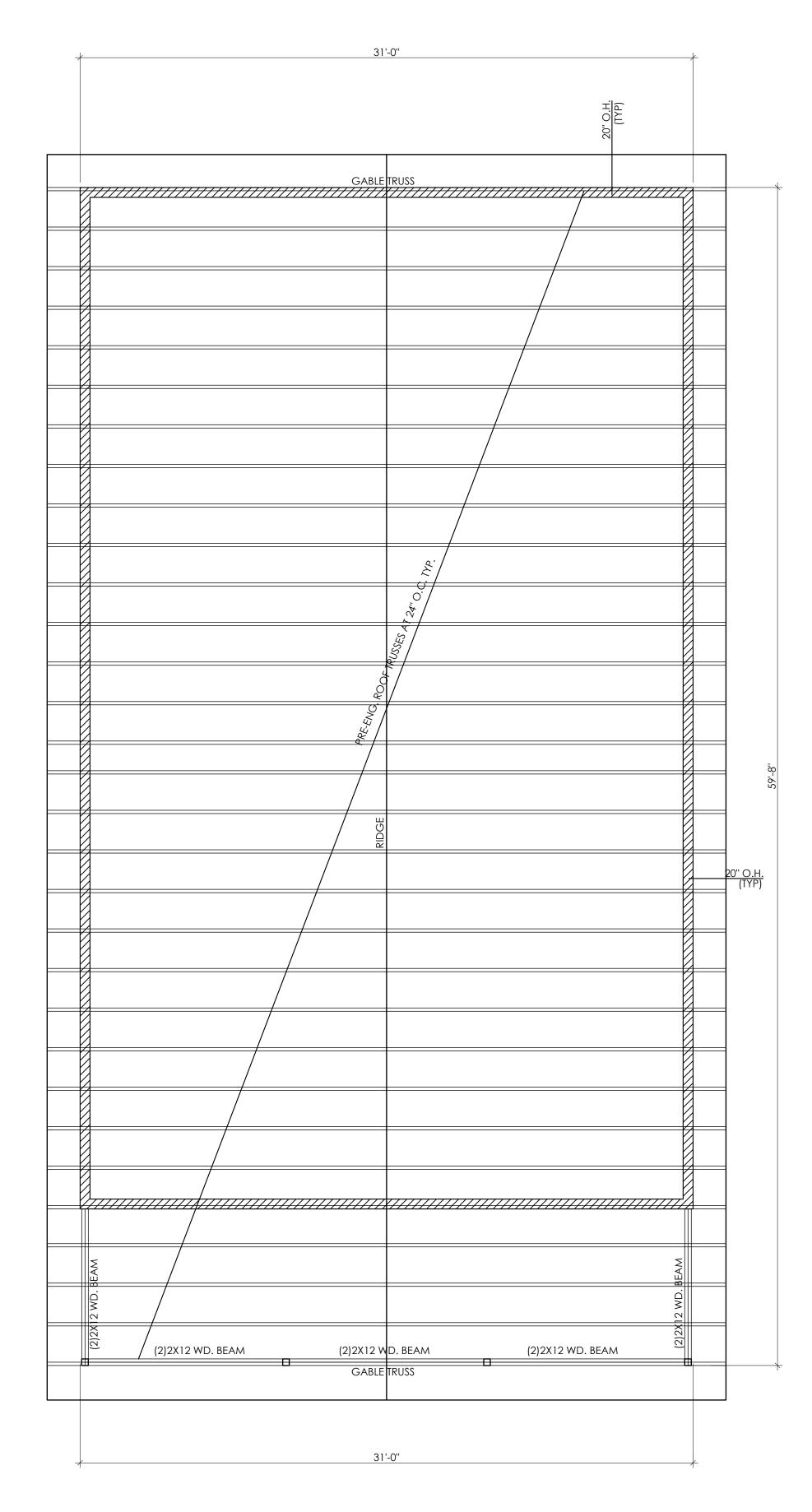
PLAN APPROVAL

ELEVATIONS

Sheet

Right Side Elevation
Scale: 1/4" = 1'-0"

HARDIE SIDING



Roof Framing Plan
Scale: 1/4" = 1'-0"

BY TRUSS MANUFACTURER. TRUSS MANUFACTURER SHALL HAVE THE AUTHORITY TO MAKE SUBSTITUTIONS FOR PRODUCTS SPECIFIED ON THE PLANS DUE TO AVAILABILITY OR ECONOMICS. CHANGES SPECIFIED BY THE TRUSS MANUFACTURER SHALL CONTROL. CHANGES MADE AFTER TRUSS ENGINEERING HAS BEEN PROVIDED TO ENGINEER OF RECORD, MUST BE APPROVED BY THE ENGINEER OF RECORD.

FRAMING PLAN IS DIAGRAMMATIC IN NATURE AND IS PROVIDED FOR ILLUSTRATION PURPOSES ONLY. TRUSS MANUFACTURER TO PROVIDE SEPARATE LAYOUT AND TRUSS COMPONENT DESIGN SIGNED AND SEALED BY A FLORIDA REGISTERED PROFESSIONAL ENGINEER.

ALL PRE-ENGINEERED WOOD PRODUCTS ARE THE RESPONSIBILITY OF THE TRUSS MANUFACTURER. THE TRUSS ENGINEER IS A DELEGATED ENGINEER FOR THIS PROJECT, AND AS SUCH, IS RESPONSIBLE FOR THE VALIDITY OF THE COMPONENTS PROVIDED. FRAMING LAYOUTS SHOWN MAY BE CHANGED BY THE TRUSS MANUFACTURER. THE DELEGATED ENGINEER IS RESPONSIBLE FOR PROVIDING A FINAL SEALED SET OF ALL CALCULATIONS AND LAYOUTS FOR THIS PROJECT TO THE ENGINEER OF RECORD FOR REVIEW PRIOR TO MANUFACTURE OF SAID COMPONENTS. ENGINEER OF RECORD HAS NOT REVIEWED THE PRE-ENGINEERED TRUSS MANUFACTURER'S COMPONENTS AT THIS TIME AND RESERVES THE RIGHT TO MAKE ANY CHANGES AFTER SUCH INFORMATION HAS BEEN PROVIDED FOR REVIEW. CONTRACTOR, AS PROJECT COORDINATOR, SHALL BE RESPONSIBLE FOR INSURING INFORMATION REQUESTED ABOVE HAS BEEN SUBMITTED TO ENGINEER OF RECORD IN A TIMELY MANNER WHEN AVAILABLE.

ALL PRE-ENGINEERED TRUSSES TO BE DESIGNED USING THE MOST RECENT TPI CRITERIA. TRUSSES TO BE HANDLED AND INSTALLED USING MOST RECENT BCSI RECOMMENDATIONS. TEMPORARY AND PERMANENT BRACING SHALL BE PER MOST RECENT BCSI RECOMMENDATIONS UNLESS NOTED OTHERWISE, OR MORE STRINGENT CODE REQUIREMENTS APPLY. TRUSS ENGINEER IS RESPONSIBLE FOR INDICATING ALL TRUSS TO TRUSS CONNECTORS. ALL COMPONENTS TO BE DESIGNED FOR BOTH GRAVITY AND UPLIFT LOAD CASES, INCLUDING BEAM COMPONENTS.

UPON REVIEW, ENGINEER OF RECORD WILL PROVIDE A REVIEW LETTER INDICATING ANY CHANGE IN STRAPPING OR SUPPORT BASED ON THAT REVIEW.

CONSTRUCTION COMMENCING PRIOR TO ENGINEER'S REVIEW IS SUBJECT TO MODIFICATION BASED ON REVIEW LETTER.

CONNECTOR NOTES: UNLESS NOTED OTHERWISE:

ALL MASONRY TO TRUSS CONNECTIONS SHALL BE SIMPSON META 16 OR EQUAL.

ALL FRAME TO TRUSS CONNECTIONS SHALL BE SIMPSON MTS16 (1 OR 2 PLY)

CONNECTOR DESIGNATIONS REFER TO THE CONNECTOR SCHEDULE ON THE "S" SHEETS

IF CONTRACTOR REQUIRES CLARIFICATION OF ANY ITEM OR COMPONENT, THEY SHALL REQUEST CLARIFICATION IN WRITING BEFORE INSTALLING ITEM IN QUESTION.

CONTRACTOR SHALL BE RESPONSIBLE FOR ITEMS INSTALLED INCORRECTLY.

NOTE: ALL LOAD BEARING WOOD

HEADERS TO BE (2) 2"x12" U.N.O.

NOTE:
REFER TO SHEET 3 FOR
FOR CONNECTOR CALL-OUTS & DETAILS.

INDICATES
LOAD BEARING WALL

GENERAL NOTES:

- 1. VERIFY ALL DIMENSIONS AND CONDITIONS IN THE FIELD PRIOR TO BEGINNING CONSTRUCTION. BRING ANY DISCREPANCIES TO THE ENGINEER'S ATTENTION PRIOR TO BEGINNING THE AFFECTED WORK.
- 2. THE STRUCTURE IS DESIGNED TO BE SELF-SUPPORTING AND STABLE AFTER THE BUILDING IS COMPLETE. THE CONTRACTOR IS RESPONSIBLE FOR DETERMINING ERECTION PROCEDURES, SEQUENCING, AND TIMING TO INSURE THE SAFETY OF THE BUILDING AND IT'S COMPONENTS DURING CONSTRUCTION. THIS INCLUDES ANY ADDITIONAL SHORING OR BRACING.

DESIGN CRITERIA:
ACI 318-08 - STRUCTURAL CONCRETE

ASCE/SEI 7-10 FLORIDA BUILDING CODE 2014, 5th EDITION

 COMPONENT AND CLADDING PRESSURES

 DESIGN LOADS:
 ZONE 1
 +32.73 PSF
 -35.76 PSF

 FLOOR
 ZONE 2
 +32.73 PSF
 -41.83 PSF

 LIVE: 40 PSF
 ZONE 3
 +32.73 PSF
 -41.83 PSF

 DEAD: 10 PSF
 ZONE 4
 +35.76 PSF
 -38.79 PSF

 ZONE 5
 +35.76 PSF
 -47.89 PSF

 ZONE 2H
 +27.28 PSF
 -60.62 PSF

 ROOF
 ZONE 3H
 +27.28 PSF
 -60.62 PSF

LIVE: 20 PSF
DEAD: 10 PSF + TRUSS SYSTEM SELF WEIGHT
WIND UPLIFT RESISTANCE: 0 PSF

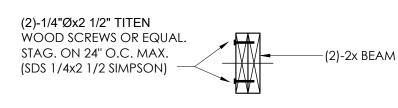
RISK CATEGORY: II WIND SPEED: 140 MPH (ULT.), 108 MPH (NOM.)

EXPOSURE: B
ENCLOSED BUILDING-ALL OPENINGS WIND-BORNE DEBRIS RATED
INTERNAL PRESSURE COEFFICIENTS: +/- 0.18 (GCpi)
MAXIMUM WINDOW PRESSURE (ZONE 5): -47.89 PSF
ASSUMED FINISHED FLOOR ELEVATION TO BE AT 0'-0" (RELATIVE) UNLESS
OTHERWISE NOTED.

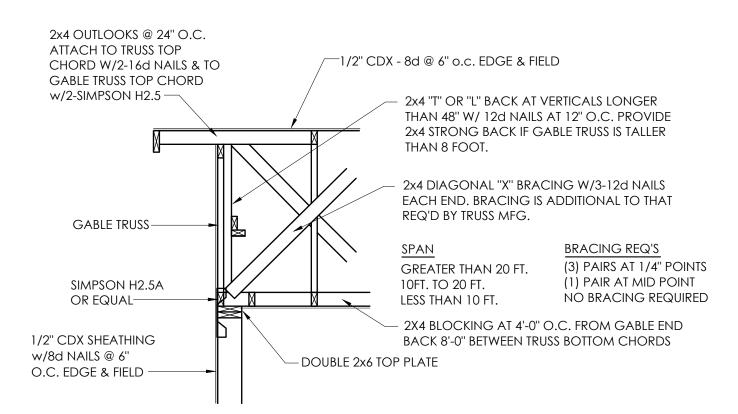
- FOUNDATION DESIGN OF THIS STRUCTURE IS BASED ON AN
 ASSUMED MINIMUM ALLOWABLE SOIL CONTACT PRESSURE OF 2000
 PSE
- 2. BOTTOMS OF ALL FOUNDATIONS AND FOOTINGS SHALL BE A MINIMUM OF 12" BELOW ADJACENT FINISHED GRADE AND FOUNDATIONS AND SLABS SHALL REST ON PROPERLY PREPARED SOIL.

MATERIALS AND CONSTRUCTION

- 1. CONCRETE fc SHALL EQUAL OR EXCEED 3000 PSI AT 28 DAYS. SLUMP SHALL BE 6"-11". SLABS SHALL BE WET-CURED. CONCRETE EXPOSED TO EARTH SHALL HAVE A COVER OF 3" FOR REINF.
- 2. MASONRY UNITS SHALL CONFORM TO ASTM C90 WITH A MINIMUM I'M OF 1500 PSI. MORTAR SHALL BE TYPE "M" OR "S" AND CONFORM TO ASTM C270. MASONRY GROUT (FOR FILLING CELLS) SHALL CONFORM TO ASTM C476 WITH A MINIMUM IC OF 3000 PSI AND A SLUMP OF 8"-11". MAXIMUM SPACING BETWEEN GROUTED CELLS SHALL NOT EXCEED 72" O.C. (AND AT CORNERS AND BELOW FRONT AND BACK PORCH COLUMNS). FILL ALL CELLS BELOW GRADE.
- 3. REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60, DEFORMED BARS. BARS SHALL BE MANUFACTURED, BENT, STORED, PLACED, SUPPORTED AND TIED ACCORDING TO APPLICABLE ACI STANDARDS CHAIRS OR BOLSTERS SHALL BE USED TO SUPPORT ALL REINFORCING. PROVIDE MINIMUM LAP LENGTHS OF 36 BAR DIAMETERS FOR HORIZONTAL REINFORCING AND CORNER AND TEE BARS. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185. LAP WWF 1 SPACE + 2".
- 4. ALL TIMBER AGAINST CONCRETE OR EARTH TO BE PRESSURE TREATED SOUTHERN PINE NO. 2.



MULTI-PLY CONN.



GABLE END BRACING DETAIL

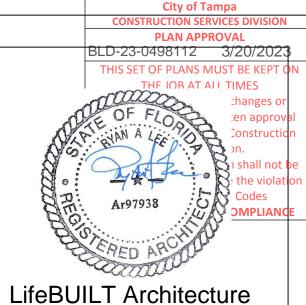
ROOFING INSTALLATION NOTES

- 1. ALL SHINGLES SHALL BE MINIMUM 25 YEAR DIMENSIONAL SHINGLES. CLASS A RATED FROM UL & PASS 997 WIND TEST CSA A 1235 & CSA 1235-98, ASTM D 3018 TYPE 1, ASTM D3161 TYPE 1, ASTM D3462.
- 2. 141 MPH WIND LOADING ON SHINGLES APPLIES ONLY WHEN SHINGLES ARE INSTALLED USING (4) NAILS PER SHINGLE & PRODUCT IS INSTALLED WITH RIDGE CAP SHINGLES.
- 3. ROOFING SYSTEM SHALL BE INSTALLED ON PRE-MANUF. PRE-ENGINEERED WOOD TRUSS SYSTEM @ 24" O.C. (U.O.N.)
- 4. ROOFING SYSTEM SHALL COMPLY WITH THE 2014 FBC. MATERIALS & INSTALLATION REQUIREMENTS FOR THE WIND SPEED SPECIFIED IN THE GENERAL NOTES
- 5. ROOF DECKING SHALL BE MIN. 1/2" PLYWOOD OR 1/2" OSB SHEATHING NAILED WITH 8d NAILS 6" O.C. @ EDGE & 6" O.C. FIELD.
- 6. UNDERLAYMENT SHALL CONSIST OF MIN. 15Ib FELT ON ROOF DECKING PER ASTM D226 TYPE I OR ASTM D4869 TYPE I. ANY SLOPE LESS 4:12 REQUIRES 2 LAYERS.
- 7. ALL METAL ROOFING SHALL COMPLY WITH THE 2014 FBC (REFER TO CONSTRUCTION & MANUF.'S DWGS.)
- 8. ALL TILE ROOFING SHALL COMPLY WITH 2014 FBC. (REFER TO CONSTRUCTION & MANUF.'S DWGS.)
- 9. ALL FLASHING, CRICKETS & DRIP EDGES SHALL COMPLY WITH THE 2014 FBC. (SEE CONST. DWGS.)
- 10. ROOFING CONTRACTOR SHALL PROVIDE ONE PACKAGE OF ROOFING TO BE LEFT ON JOB SITE FOR FINAL ROOFING INSPECTION FOR PROOF OF COMPLIANCE.

WINDOW & DOOR INSTALLATION NOTES

- 1. WINDOWS & DOORS SHALL BE INSTALLED FOR THE WIND LOADS SPECIFIED IN THE GENERAL NOTES.
- 2. ALL CUT SHEETS, INSTALLATION DETAILS, INSTALLATION SPECIFICATIONS & NOTES FOR ALL WINDOWS & DOORS SHALL BE PROVIDED BY WINDOW/DOOR MANUFACTURER AT TIME OF PERMITTING AND SHALL ACCOMPANY CONSTRUCTION DWGS.
- 3. ALL WINDOWS & DOORS SHALL COMPLY TO THE 2014 FBC.
- 4. WINDOWS & DOORS SHALL BE SHIMMED AS REQ'D. AT EACH ANCHOR WITH LOAD BEARING SHIMS FOR SPACES GREATER THAN 1/16" MAXIMUM 1/4" SHIM. ALL ANCHORS TO BE 3/16" DIA. CONC. SCREW WITH MIN EMBED. 1 1/4" INTO MASONRY OR CONC.
- 5. ALL WINDOWS SHALL BEAR LABELS SHOWING COMPLIANCE WITH ANSI/AAMA/NWWD A101/IS 2-97 STD. & COMPLIANCE w/THE 2014
- 6. SLIDING GLASS DOORS SHALL BE SHIMMED AS REQ'D. AT EACH ANCHOR WITH LOAD BEARING SHIMS FOR SPACES GREATER THAN 1/16" MAXIMUM 1/4" SHIM. ALL ANCHORS TO BE 3/16" DIA. CONC. SCREWS WITH MIN. EMBED. 1 1/4" INTO MASONRY OR CONC. HEAD & SILL TO HAVE 5 ANCHORS & EACH JAMB TO HAVE 6 ANCHORS PER MANUF. RECOMMENDATIONS. ALL HOLES SHALL BE PRE-DRILLED.

	FLORIDA	PRODUCT AF	PROVAL	LISTING	
PRODUCT CATEGORY	SUB-CATEGORY	MANUFACTURER	TYPE	NUMBER	ITEM DESCRIPTION
STRUCTURAL COMPONENT	WOOD CONNECTORS	SIMPSON STRONG-TIE	ANCHORS	FL 10456-R3	W.D. CONNECTORS & ANCHORS
STRUCTURAL COMPONENT	WOOD CONNECTORS	SIMPSON STRONG-TIE	ANCHORS	FL 10531-R3	W.D. CONNECTORS & ANCHORS
STRUCTURAL COMPONENT	WOOD CONNECTORS	SIMPSON STRONG-TIE	ANCHORS	FL 10852-R3	W.D. CONNECTORS & ANCHORS
STRUCTURAL COMPONENT	WOOD CONNECTORS	SIMPSON STRONG-TIE	ANCHORS	FL 10860-R3	W.D. CONNECTORS & ANCHORS
STRUCTURAL COMPONENT	WOOD CONNECTORS	SIMPSON STRONG-TIE	ANCHORS	FL 10866-R3	W.D. CONNECTORS & ANCHORS
STRUCTURAL COMPONENT	WOOD CONNECTORS	SIMPSON STRONG-TIE	ANCHORS	FL 11470-R3	W.D. CONNECTORS & ANCHORS
STRUCTURAL COMPONENT	WOOD CONNECTORS	SIMPSON STRONG-TIE	ANCHORS	FL 11473-R3	W.D. CONNECTORS & ANCHORS
STRUCTURAL COMPONENT	WOOD CONNECTORS	SIMPSON STRONG-TIE	ANCHORS	FL 13872-R2	W.D. CONNECTORS & ANCHORS
STRUCTURAL COMPONENT	WOOD CONNECTORS	SIMPSON STRONG-TIE	ANCHORS	FL 13904-R3	W.D. CONNECTORS & ANCHORS
STRUCTURAL COMPONENT	WOOD CONNECTORS	SIMPSON STRONG-TIE	ANCHORS	FL 2355-R5	W.D. CONNECTORS & ANCHORS
STRUCTURAL COMPONENT	NEW TECHNOLOGY	POWERS STEEL	LINTELS	FL 11383-R4	STEEL LINTEL
STRUCTURAL COMPONENT	NEW TECHNOLOGY	CAST-CRETE	LINTELS	FL 158-R7	CONCRETE PRODUCTS
ROOFING	ROOF SHINGLES	GAF MATERIAL, INC.		FL 10124-R15	ASPHALT SHINGLES
ROOFING	ROOF SHINGLES	GAF MATERIAL, INC.		FL 16730-R5	SINGLE PLY ROOFING
ROOFING	ROOF SHINGLES	GAF MATERIAL, INC.		FL 10626-R12	UNDERLAYMENT
ROOFING	ROLLED ROOFING	GAF MATERIAL, INC.		FL 11946-R7	BUILT-UP ROOFING
ROOFING	ROOF SHINGLES	GAF MATERIAL, INC.		FL 16732-R4	MODIFIED BITUMEN
ROOFING	METAL ROOFING	GULF COAST SUPPLY		FL 11651-R2	METAL ROOFING
ROOFING	TILE	MONIER LIFETILE		FL 601-R10	CONC. ROOF TILE
DOORS	GARAGE DOORS	OVERHEAD DOOR CORP.		FL 742-R7	ROLL UP
DOORS	GARAGE DOORS	OVERHEAD DOOR CORP.		FL 14170-R7	SECTIONAL DOORS
DOORS	GARAGE DOORS	OVERHEAD DOOR CORP.		FL 16798-R2	SECTIONAL DOORS
DOORS	GARAGE DOORS	CLOPAY		FL 5684-R7	SECTIONAL DOORS
DOORS	GARAGE DOORS	CLOPAY		FL 5678-R3	SECTIONAL DOORS
DOORS	EXTERIOR DOORS	JELD-WEN	IMPACT	FL 11112.1-R4	SWING DOORS
	EXTERIOR DOORS		IMPACT		
DOORS	EXTERIOR DOORS	JELD-WEN	IMPACT	FL 12796-R6	SLIDING DOORS
WINDOWS	WINDOWS	JELD-WEN	IMPACT	FL 11120-R11	SINGLE HUNG
WINDOWS	WINDOWS	JELD-WEN	IMPACT	FL 14087.2-R6	FIXED
PANEL WALLS	SOFFIT	PETERSON ALUMINUM CORP.	ALUMINUM	FL 4483-R7	SOFFIT
PANEL WALLS	SOFFIT	CERTAINTEED CORP.	VINYL	FL 13389-R2	SOFFIT
PANEL WALLS	SIDING	JAMES HARDIE BUILDING PRODUCTS, IN.C	FIBER CEMENT	FL10477-R3	LAP SIDING
PANEL WALLS	SIDING	JAMES HARDIE BUILDING PRODUCTS, IN.C	FIBER CEMENT	FL13192-R4	PLANK LAP SIDING
PANEL WALLS	SIDING	JAMES HARDIE BUILDING PRODUCTS, IN.C	FIBER CEMENT	FL13223-R3	PANEL SIDING



SET DATE

OWNER

ADAM DUFF

ADDRESS

105 WEST HILDA AVE TAMPA, FL 33603

DRAWINGS BY

LESLIE SIMPSON 404 414 7850 Isimpson@lgsarchdesign.com

CONTRACTOR

ANDRES PENA 813 470 8323 apena9@me.com

ENGINEER

N/A

DATE:

DECEMBER 2022

PROJECT NAME

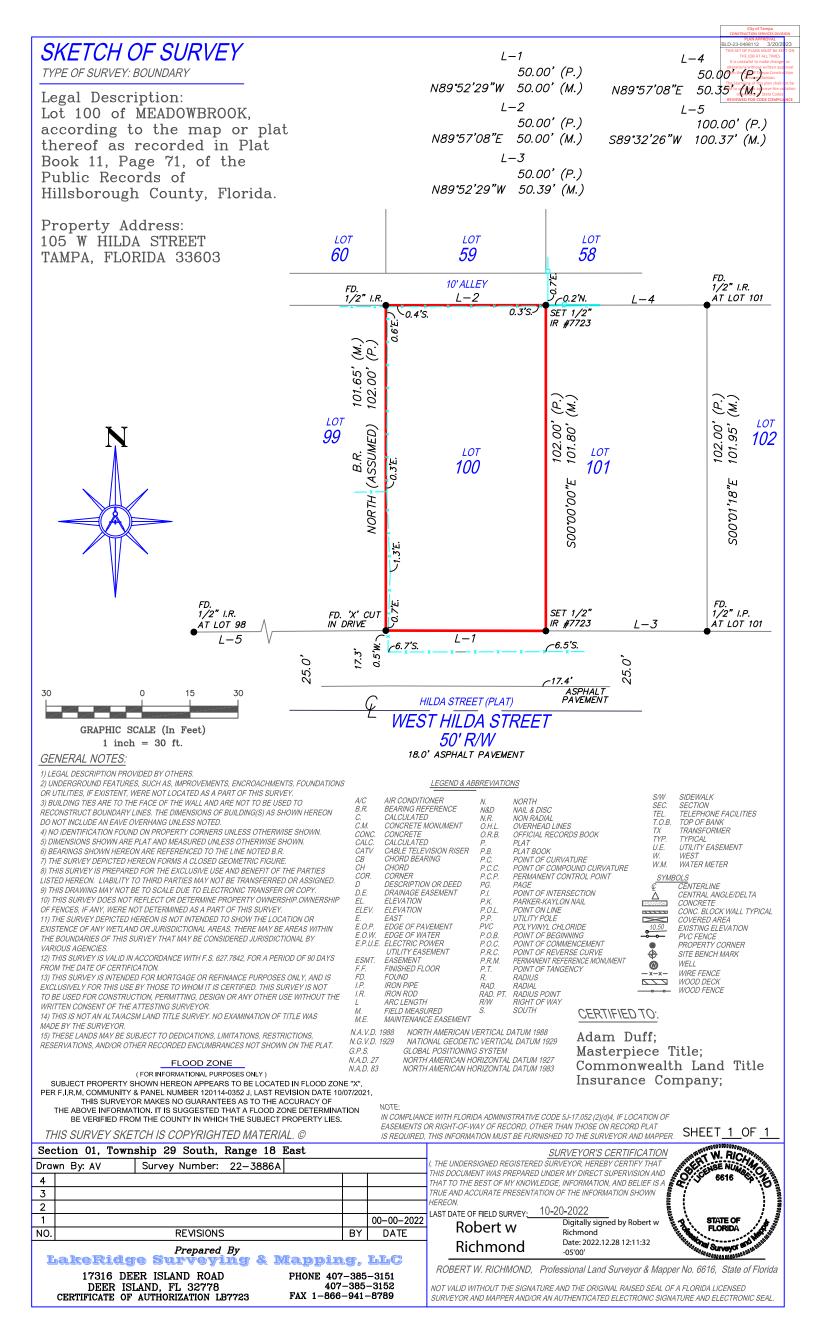
NEW HOME

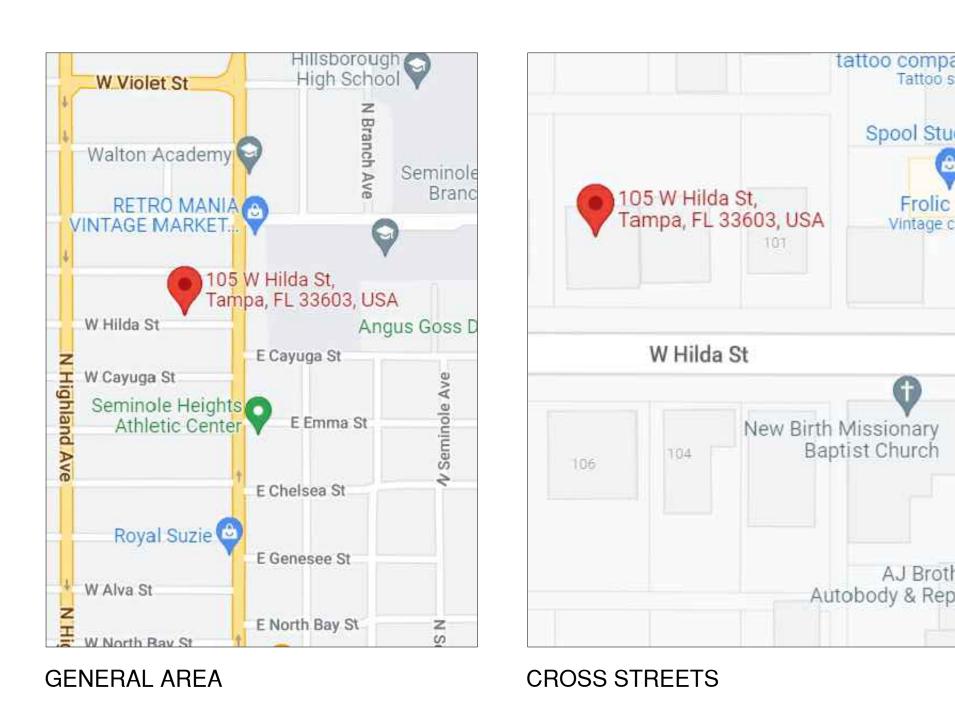
105 W HILDA

OOF FRAMING PLAN

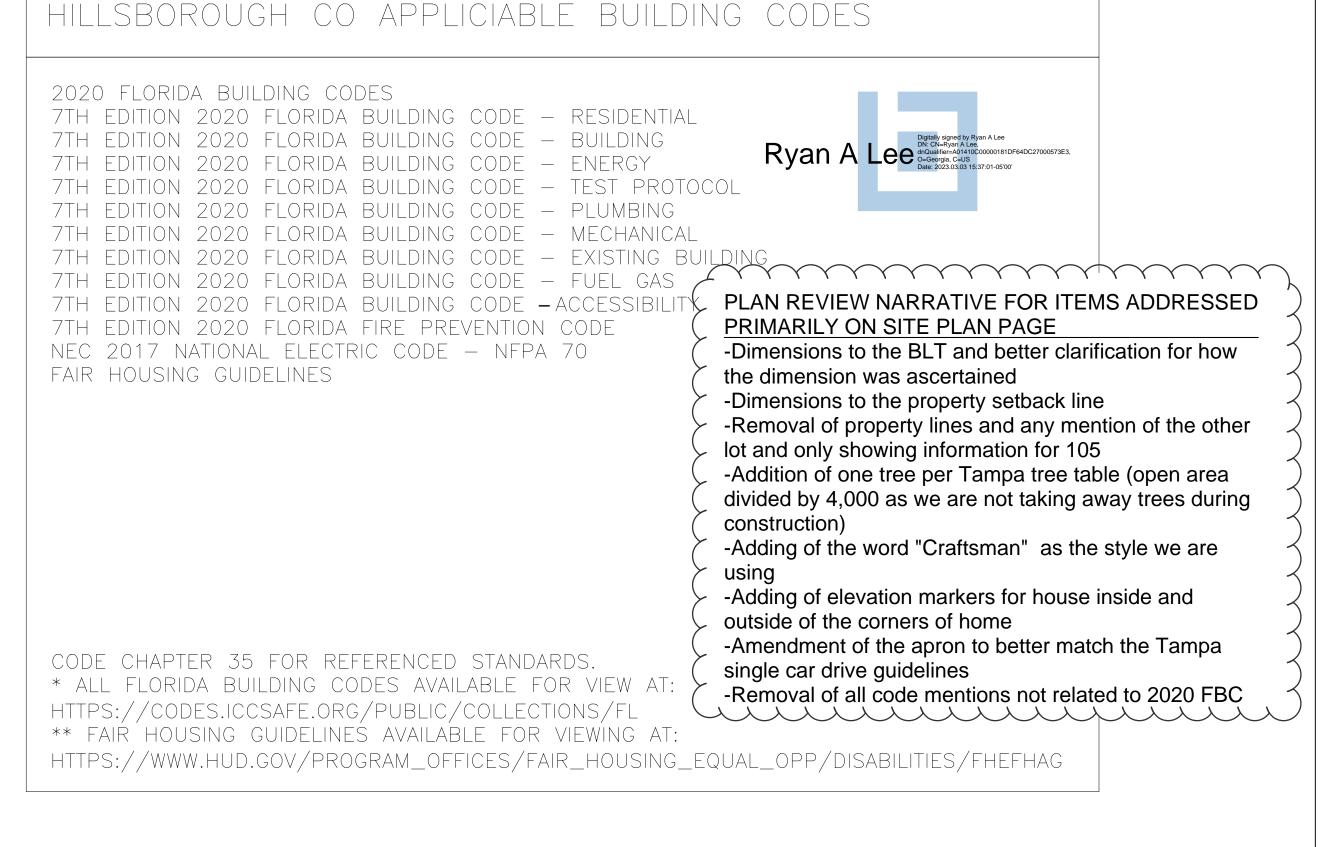
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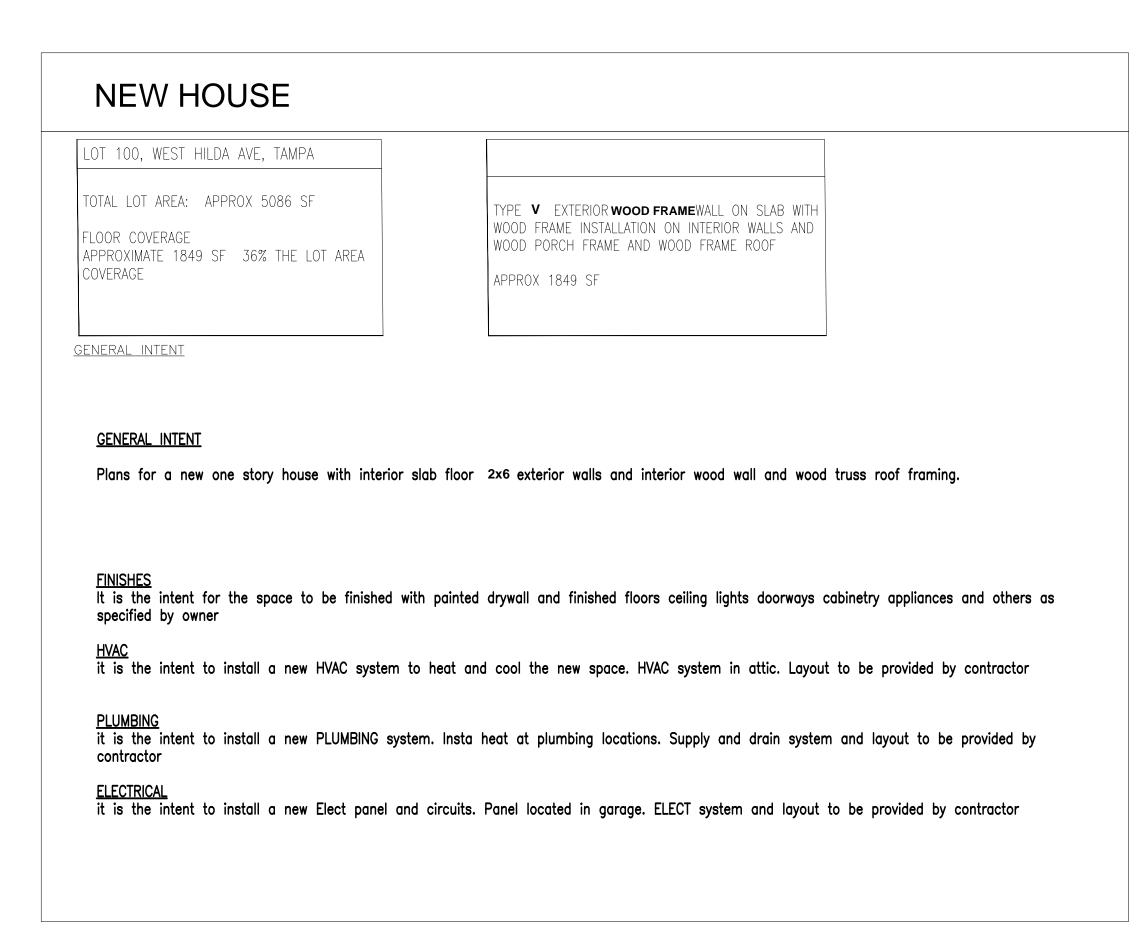




FRONT ELEVATION

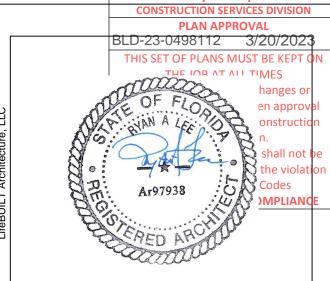
CRAFTSMAN STYLE

105 WEST HILDA AVENUE



NEW HOME CONSTRUCTION

TAMPA FL 33603



SET	DATE
01	02.06.23

OWNER

ADAM DUFF

ADDRESS

105 WEST HILDA AVE TAMPA, FL 33603

DRAWINGS BY

LESLIE SIMPSON 404 414 7850 Isimpson@lgsarchdesign.com

CONTRACTOR

ANDRES PENA 813 470 8323 apena9@me.com

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NEW HOME

105 W HILDA

COVER

Sheet

A 0.0